

**DESIGN CALCULATION AND DRAWING FOR  
TRANSVERSE GUIDED BEARING  
(Ch. 516+938)**

**PROJECT:** FOUR LANING OF JHANJHI TO DEMOW SECTION OF NH-37 FROM EXISTING CH. K 491+050 TO KM 535+250  
(DESIGN CH. KM 4900+800 TO KM 534+800) IN THE STATE OF ASSAM UNDER EPC MOD

**CLIENT:** NATIONAL HIGHWAYS & INFRASTRUCTURE DEVELOPMENT CORPORATION LTD. (NHIDCL)

**CONTRACTOR :** M/S KAMAC-SHIVA HARLALKA (JV)

**MANUFACTURER:**



**M/S KARMA ENTERPRISE, GUWAHATI, ASSAM**

**Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing**

TYPE OF BEARING :  
REVISION :

TG  
00

TYPE: B2  
CH: 516+938

Conc. Grade for Pedestal M 45  
Conc. Grade for Superstructure M 50

**DESIGN DATA FOR BEARING DESIGN (LOADS)**

LOADING PARAMETERS	SLS			ULS	
UNITS	MT		kN	MT	kN
VERTICAL MAXIMUM LOAD	167.94	1647.00		241.16	2365.00
VERTICAL MINIMUM LOAD	68.63	673.00		117.27	1150.00
VERTICAL PERMANENT LOAD	85.96	843.00			
HORIZONTAL LOADS					
ACTING IN LONGITUDINAL DIRECTION	5.10	50.00		7.65	75.00
ACTING IN TRANSVERSE DIRECTION	0.00	0.00		0.00	0.00
RESULTANT HORIZONTAL LOADS		50.00			75.00
DISPLACEMENT					
LONGITUDINAL (MM)	0.00	0.00		0.00	
TRANSVERSE (MM)	+ 20.00	-20.00		40.00	
ROTATION (RADIAN)	0.0038	+	0	=	0.004

Assume Permanent Rotation       $q_p = 0.41$        $x = 0.004$        $= 0.002$   
Assume Variable Rotation       $q_v = 0.59$        $x = 0.004$        $= 0.002$

**MATERIALS**

Steel Stress (Working) for Design Use      340 MPa (Grade-340-570W)      AS PER IS - 1030  
HT Bolts shall conform to Grade 8.8 of IS : 1364  
Elastomer shall be of hardness 50 +/- 5 conforming to IRC : 83 (Part III) - 2018 Table - 4.3

**DIMENSION DETAILS OF BEARING COMPONENTS**

**ELASTOMERIC PAD**

PAD DIAMETER      =       $d_i$       325 mm  
PAD THICKNESS      =       $h_e$       24 mm

**PTFE**

PTFE DIAMETER      =       $D_{ptfe}$       325 mm  
PTFE THICKNESS      =       $T_{ptfe}$       5 mm

**CYLINDER**

CYLINDER CONCRETE CONTACT DIAMETER      =       $D_o$       395 mm  
CYLINDER BASE THICKNESS      =       $k_b$       20 mm  
CYLINDER INNER DIAMETER      =       $D_i = d_i$       325 mm  
CYLINDER OUTER DIAMETER      =       $d_o$       375 mm  
HEIGHT OF CYLINDER      =       $h_c$       36 mm  
WALL THICKNESS      =       $T_{cw}$       25.0 mm

**INTERMEDIATE COMPONENT**

LENGTH      =       $L_{ic}$       365 mm  
WIDTH      =       $W_{ic}$       365 mm  
INTERMEDIATE COMPONENT ABOVE THICKNESS      =       $T_p$       12 mm  
INTERMEDIATE COMPONENT PROJECTION      =       $h_p$       22 mm  
VERTICAL FACE      =       $w$       6 mm

**TOP PLATE**

EFFECTIVE CONCRETE CONTACT DIAMETER      =       $D_{tpeff}$       375 mm  
LENGTH      =       $L_t$       420 mm  
WIDTH      =       $B_t$       410 mm  
THICKNESS      =       $T_t$       22 mm  
S/S SHEET LENGTH      =       $L_{ss}$       410 mm  
S/S SHEET WIDTH      =       $B_{ss}$       370 mm  
S/S SHEET THICKNESS      =       $T_{ss}$       3 mm  
GUIDE BAR LENGTH      =       $L_{gb}$       420 mm  
GUIDE BAR WIDTH      =       $k_u$       15 mm  
GUIDE BAR HEIGHT      =       $H_{gb}$       18 mm

**Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing**

**ANCHORAGE**

BOLTS DIAMETER	=	Dbolt	16 mm
BOLTS LENGTH	=	Lbolt	35 mm
BOLTS PER COMPONENT	=	Nbolt	4 NOS.
GRADE OF BOLTS	=	GR.bolt	8.8
ANCHORAGE COLLOR LENGTH	=	CL	56 mm
ANCHORAGE COLLOR THICKNESS	=	C thk	12 mm
SLEEVE LENGTH ( Superstructure)	=	Ls	110 mm
SLEEVE DIAMETER ( Superstructure)	=	Ds	40 mm
SLEEVE LENGTH ( Pedestal)	=	Lp	120 mm
SLEEVE DIAMETER ( Pedestal)	=	Dp	40 mm

**GENERAL**

NO. OF BRASS SEALING RINGS	=	Nbr	2 NOS.
TOTAL THICKNESS OF RINGS	=	Tbr	4.0 mm
GAP BETWEEN CYLINDER & TOP COMPONENT	=	h4	10 mm
<b>TOTAL BEARING ASSEMBLY HEIGHT</b>	=	<b>HT</b>	<b>105.2 mm</b>

**Calculation for Permissible Stresses in Pedestal Concrete**

**BOTTOM**

Cylinder Concrete Contact Diameter =		395	mm
Loaded area ( $A_{c1} = \pi \times Db^2 / 4$ ) =		122541.75	mm <sup>2</sup>
Required Pedestal Size for Dispersion =	790.00	x	790 mm
Dispersed area ( $A_{c1} = \pi \times d^2 / 4$ ) =		490167	mm <sup>2</sup>

**TOP**

Top Component Contact Diameter =		375	mm
Loaded area ( $A_2 = \pi \times Db^2 / 4$ ) =		110446.62	mm <sup>2</sup>
Required Superstructure Size for Dispersion =	750.00	x	750 mm
Dispersed area ( $A_1 = \pi \times d^2 / 4$ ) =		441786.47	mm <sup>2</sup>

**DESIGN CALCULATIONS :-**

REF. CODE : IRC:83 (Part-III)-2018

**1.1 DESIGN OF PAD (Clause - 5.2.3.2)**

Effective diameter of Pad	Dpad		325 mm
Area of pad = $\pi \times d^2 / 4$	a		82991.0714 mm <sup>2</sup>
Vertical Load	Nsd	2365.00	kN
Direct Pressure Nsd / a	pa	28.50	N/mm <sup>2</sup>
(Nsd / a ) x Ym		37.05	N/mm <sup>2</sup>
fe,k		60.00	N/mm <sup>2</sup>
		<b>OK</b>	

**Check Compression at edge of Neoprene Pad (Clause - 5.2.3.4)**

Max. Permitted = 15 % of hc		3.60	mm
Desired Rotation		0.00380	radians
Available Rotation in Radius due to Compression of Pad		0.02215	radians
			<b>OK</b>
Diameter / Thickness Ratio	13.54	Maximum (Dpad/hc)	15
			<b>OK</b>

**Check for Min. average Stress (Clause - 5.2.3.3)**

Min. average stress = (Nsd min. / a)		8.11	N/mm <sup>2</sup>
Permissible Min. average stress =		2.00	N/mm <sup>2</sup>
		<b>OK</b>	

### Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing

#### Pressure on PTFE

Diameter of PTFE	L	325.00	mm
Area of PTFE	Aco	82957.68	mm <sup>2</sup>
Average pressure on PTFE = $N_{max} ULS / A_p$	$\sigma_p$	28.51	N/mm <sup>2</sup>
Protrusion of PTFE			
$h = 1.75 + L / 1200$		2.02	mm
h provided		2.20	mm
Thickness of PTFE = 2.2h		4.84	mm
Thickness of PTFE provided		5.00	mm
		<b>OK</b>	
Characteristic compressive strength of PTFE	$f_k$	90.00	N/mm <sup>2</sup>
The Characteristic compressive strength is reduced due to exceeding the category temperature above 30°C.			
Max. ambient temperature		40.00	°C
Reduction		20.00	%
Reduction factor	k	0.80	
$\mu_{max} = 1.2 / (10 + \sigma_p)$		0.031	
	0.03	≤	0.031
$e1 = \mu_{max} \times R$		5.06	mm
$e3 = \sigma_d \times L / 2$		0.62	mm
$e = e1 + e3$		5.68	mm
	L/8	40.63	mm
		<b>OK</b>	
$\lambda = (1 - 0.75 \times \pi \times e / L)$	$\lambda$	0.96	
Reduced contact area $A_r = A_{co} \times \lambda$	$A_r$	79540.77	mm <sup>2</sup>
$\max, N_{sd} = A_r \times k \times f_k / \gamma_m$	$\gamma_m = 1.4$	4090.67	kN
$N_{sd}, ULS$		2365.00	kN
		<b>OK</b>	

#### STRESS IN BOTTOM CONCRETE

Bottom dispersion width	$D_o$	395	
Thickness of Bottom Plate	$k_b$	20	
Area of Bottom Dispersion = $p \times d^{2/4}$	a	122591.071	mm <sup>2</sup>
Section Modulus = $p \times d^{3/32}$	Z	6052934.152	mm <sup>3</sup>
Vertical Load	$N_{sd}$	2365.00	kN
Horizontal force	$V_{sd}$	75.00	kN
Moment of resistance due to rotation:			
Rotation due to dead load	$\theta_p$	0.00155	radians
Live Load	$\theta_v$	0.00225	radians
Ratio = $d_i / h_e$		13.54	
For induced moment	$k1$	1.97	
	$k2$	76.38	
Induced moment due to rotation $M_{e,d}$	$M_{e,d}$	5997.30	kN-mm
$d_i^3 \times (k1 \times q_p + K2 \times q_v) / 1000$			
Moment of resistance due to HF:			
Horizontal distance	C	162.50	mm
Resultant HF	$V_{sd}$	75.00	kN
Moment of resistance due to HF:	$M_{r,d}$	2437.50	kN-mm
$0.2 \times C \times V_{sd}$			
Total Movement = $M_{e,d} + M_{r,d}$	$M_t$	8434.80	kN-mm
Direct Pressure $N_{sd} / a$	pa	19.29	N/mm <sup>2</sup>
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	20.10	N/mm <sup>2</sup>
		<b>OK</b>	
Bending Stress $M_t / Z$	pb	1.39	N/mm <sup>2</sup>
Permissible bending stress		14.85	N/mm <sup>2</sup>
		<b>OK</b>	
Area on Pedestal	$A_{c1}$	490167.00	mm <sup>2</sup>
$F_{rdu} = A_{co} \times f_{cd} \times (\sqrt{A_{c1} / A_{c0}})$		4926.18	kN
$3 \times f_{cd} \times A_{co}$		7389.27	kN
$F_{rdu} \leq 3 \times f_{cd} \times A_{co}$		<b>OK</b>	



**Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing**  
**STRESSING IN TOP CONCRETE**

<b>Top dispersion width</b>		<b>Dtpeff</b>	<b>375</b>
<b>Thickness of Top Plate</b>		<b>Tt</b>	<b>22</b>
Area of Top Dispersion = $p \times d^{2/4}$	a	110491.071	mm <sup>2</sup>
Section Modulus = $p \times d^{3/32}$	Z	5179268.973	mm <sup>3</sup>
Vertical Load	Nsd	2365.00	kN
Horizontal force	Vsd	75.00	kN
Moment of resistance due to rotation:			
Rotation due to dead load	$\theta_p$	0.00155	radians
Live Load	$\theta_v$	0.00225	radians
Ratio = $d_i / h_e$		13.54	
For induced moment	k1	1.97	
	k2	76.38	
Induced moment due to rotation	Me.d		
$d_i^3 \times (k1 \times q_p + k2 \times q_v) / 1000$		5997.30	kN-mm
Moment of resistance due to HF:			
Horizontal distance	C	162.50	mm
Resultant HF	Vsd	75.00	kN
Moment of resistance due to HF:	Mr.d	2437.50	kN-mm
$0.2 \times C \times Vsd$			
Total Movement = $Me.d + Mr.d$	Mt	8434.80	kN-mm
Direct Pressure = $Nsd / a$	pa	21.40	N/mm <sup>2</sup>
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	22.33	N/mm <sup>2</sup>
		OK	
Bending Stress $Mt / Z$	pb	1.63	N/mm <sup>2</sup>
Permissible bending stress		16.50	N/mm <sup>2</sup>
		OK	
Area on Superstructure	Ac1	441786.47	mm <sup>2</sup>
$Frd = Aco \times f_{cd} \times (\sqrt{Ac1 / Aco})$		4933.28	kN
$3 \times f_{cd} \times Aco$		7399.92	kN
$Frd \leq 3 \times f_{cd} \times Aco$		OK	
<b>Pot walls subjected to tensile force (Clause 5.3.1.2.3)</b>			
$AR = (d_o - d_i) \times h_c$	AR	1800.00	mm <sup>2</sup>
$V_{e,sd} = 4 \times Nsd \times h_e / \pi \times d_i$	$V_{e,sd}$	222.48	kN
$V_{fxy,sd}$		75.00	kN
$V_{sd} = V_{e,sd} + V_{fxy,sd}$		297.48	kN
$V_{rd} = f_y \times AR / Y_m$	$Y_m =$	556.36	kN
	1.1	OK	
<b>Pot walls subjected to shear force (Clause 5.3.1.2.4)</b>			
$V_{sd} \leq V_{re}$			
$V_{sd} = V_{e,sd} + 1.5 \times V_{fxy,sd} / d_i$		1.03	kN
$V_{re} = (f_y \times (d_o - d_i)) / (2 \times Y_m \times (\sqrt{3}))$	$Y_m =$	5.15	kN
	1.1	OK	
<b>Pot base subjected to tensile force (Clause 5.3.1.2.5)</b>			
$V_{sd} \leq V_{re}$			
$Ap = d_o \times kb$		7500.00	mm <sup>2</sup>
$V_{sd} = V_{e,sd} + V_{fxy,sd}$		297.48	kN
$V_{rd} = f_y \times Ap / Y_m$	$Y_m =$	2318.18	kN
	1.1	OK	
<b>Design resistance for integral guides</b>			
<b>Shear resistance</b>			
Thickness of guide	ku	15.00	mm
Height of guide	Hgb	18.00	mm
Length of guide	Lgb	420.00	mm
Length of side sliding	L	365.00	mm
Height of application	ha	10.50	mm
Effective length of guide	Leff	386.00	mm
$V_{sd} \leq V_{rd}$	$Y_m =$	1.1	
$V_{rd} = ku \times Leff \times f_y / \sqrt{3} \times Y_m$		1033.25	kN
$V_{sd}$		75.00	kN
		OK	

### Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing

Bending resistance in combination with shear resistance		
$P = ((2 \times V_{sd} / V_{Rd}) - 1)^2$	0.73	
$f'_{ty} = (1 - P) \times f_y$	91.55	
$M_{Rd} = k u^2 \times L_{eff} \times f'_{ty} / (4 \times Y_m)$	1807.12	kN-mm
$M_{sd} = V_{sd} \times h_a$	787.50	kN-mm
	<b>OK</b>	

#### Movement Capacity

Size of Top Plate	Lt	=	420.00	mm
	Bt	=	410.00	mm
Effective contact Concrete Diameter	Dtpeff	=	375.00	mm
Preset (If Any)		=	0.00	mm
Long. Movement Capacity (Lt - Dtpeff)		=	45.00	mm
Movement in One Direction		=	22.50	mm
Permissible Movement		=	20.00	mm
			<b>OK</b>	
Movement in Other Direction		=	22.50	mm
Permissible Movement		=	20.00	mm
			<b>OK</b>	

#### Anchor Bolts

Max Horizontal Force		75.00	kN
Min. Vertical Load		<b>1150.00</b>	kN
Frictional Force		<b>0.00</b>	kN
Diameter of Bolt		<b>16.00</b>	mm
Length of Bolt		<b>35.00</b>	mm
Number of Bolts	n	<b>4.00</b>	Nos.
Thickness of Collar	Ct	12.00	mm
Factor for Net Area	kn	0.78	
Effec. Area of Bolt ( $p \times d^2 \times \pi / 4$ )	Abolt	156.89	mm <sup>2</sup>
av		0.60	
fub		800.00	N/mm <sup>2</sup>
Ym <sup>2</sup>		1.25	
fu		570.00	N/mm <sup>2</sup>
k2		0.90	
Shear resistance $F_{v,Rd} = av \times fub \times A / Y_m^2$	$F_{v,Rd}$	60.25	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$	$F_{v,sd}$	18.75	kN
		<b>OK</b>	
Bearing resistance $F_{b,Rd} = 1.25 \times fu \times d \times t / Y_m^2$	$F_{b,Rd}$	109.44	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$		18.75	kN
		<b>OK</b>	
Tension resistance $F_{t,Rd} = k2 \times fub \times As / Y_m^2$	$F_{t,Rd}$	90.37	kN
Design tension resistance	$F_{t,sd}$	4.53	kN
Combined shear and tension $= F_{v,sd} / F_{v,Rd} + F_{t,sd} / (1.4 \times F_{t,Rd})$		0.35	
		<b>OK</b>	

#### Anchor Sleeves (Clause - 5.3.6.4.2)

<b>Top Plate</b>			
Length of Sleeve		<b>110.00</b>	mm
Diameter of Sleeve		<b>40.00</b>	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $F_{Rdu} = 1.33 D \times L \times f_{cd} / \text{sqrt}(3)$		75.46	kN
Design resistance in shear		18.75	kN
		<b>OK</b>	
<b>Bottom Plate</b>			
Length of Sleeve		120.00	mm
Diameter of Sleeve		<b>40.00</b>	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $F_{Rdu} = 1.33 D \times L \times f_{cd} / \text{sqrt}(3)$		74.08	kN
Design resistance in shear		18.75	kN
		<b>OK</b>	

**Design Calculation of SLS- 1647 KN, ULS- 2365 KN Transverse Guided Bearing**

**Requirement of clearance**

$\delta = 0.01 \times D_i$	min. = 3	3.25	<	10	mm
$h_c - h_e - (w - w_e) \times 0.5 - (\theta \times 0.5 \times D_i)$		8.60			mm
$\delta$		3.25			mm
		<b>OK</b>			
$h_p - (h_e - h_e) - (\theta \times 0.5 \times D_p)$		9.31			mm
$\delta$		3.25			mm
		<b>OK</b>			

**Curved Contact Surface** (Clause - 5.3.1.4.2)

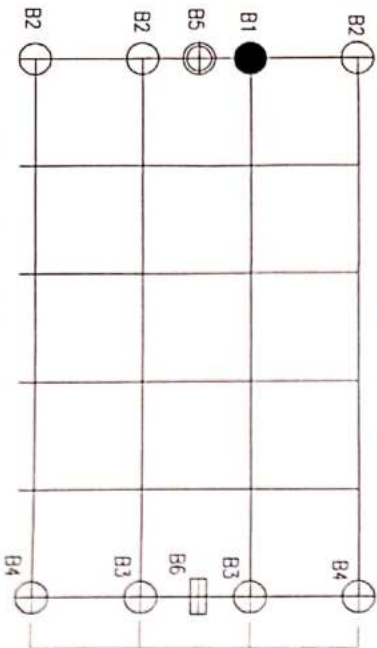
$V_{sd} \leq V_{rd}$	R	=	162.50	mm
	$f_u$	=	570.00	N/mm <sup>2</sup>
	$\theta$	=	0.004	Radian
	$\gamma_m$	=	1.10	
$V_{rd} = 15 \times f_u^{0.5} \times R \times D_i / E_s \times \gamma_m^{0.5}$		=	1063.6	kN
$V_{sd}$		=	75.00	kN
		=	<b>OK</b>	
$w_e = 3.04 \times (\text{sqrt}(1.5 \times V_{sd} \times R / E_s \times D_i))$		=	0.43	mm
$w_e + \theta \times D_i$		=	1.66	mm
Provided	w	=	6.00	mm
		=	<b>OK</b>	







BEARING TYPE & LOAD DETAILS		FIXED BEARING		LONG FIXED BEARING		TRANS FIXED BEARING		FREE BEARING		PIN BEARING		METALLIC GUIDED BEARING	
BEARING MARK		B1		B2		B3		B4		B5		B6	
TOTAL QUANTITY (NO)		1		3		1		3		1		1	
GRADE OF CONCRETE	UPPER SURFACE	M50		M50		M50		M50		M50		M50	
	SLS	M50		M50		M50		M50		M50		M50	
	SLS	M45		M45		M45		M45		M45		M45	
	ULS	M45		M45		M45		M45		M45		M45	
DESIGN LOAD (KN)	SLS	1647		1647		1647		1647		-		-	
		843		843		843		843		-		-	
		673		673		673		673		-		-	
		50		50		0		0		-		-	
TRANSLATION (MM)	SLS	0		0		0		0		-		-	
		2365		2365		2365		2365		-		-	
		1150		1150		1150		1150		-		-	
		1		1		1		1		-		0	
ROTATION (RED)	SLS	75		75		0		0		5548		2755	
		0		0		0		0		-		-	
		-		-		-		-		-		-	
		-		-		-		-		-		-	



- B1 ● FIXED BEARING
- B2 ○ LONG GUIDED BEARING
- B3 ○ LONG GUIDED BEARING
- B4 ○ LONG GUIDED BEARING
- B5 ○ LONG GUIDED BEARING
- B6 ○ LONG GUIDED BEARING

- NOTES:
- ALL DIMENSIONS ARE IN MILLIMETERS. UNLESS NOTED OTHERWISE, DIMENSIONS SHALL BE FOLLOWED.
  - THE CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF IS 456 AND IS 8000.
  - THE DESIGN OF THE BEARING SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF IS 456 AND IS 8000.
  - THE BEARING SHALL BE PROVIDED WITH A MINIMUM OF 10% OVERSTRENGTH.
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KEY PLAN SHOWING ARRANGEMENT OF BEARINGS FOR POT CUM PILE BEARINGS.